

LS-70-Revised  
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**RESPONSE TO VIBRATIONAL DISTURBANCE  
OF THE MAGNET FOUNDATION**

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Response to Vibrational Disturbance of the Magnet Foundation

Assumptions:

1. Soil under the concrete slab of uniform density and elasticity (constant  $\rho$  and constant modulus of elasticity  $E$ ).
2. Constant frictional damping.
3. No coupling between vertical and horizontal motion.

Vertical Motion

$$\text{Hooke's law: } \frac{z - z_o}{z_o} = \frac{\text{Tension}}{E} = \frac{\text{Force}}{AE}$$

$A$  = area of the slab

$$\text{Force: } F = -Mg - M \frac{d^2 z}{dt^2} - F_f + F_d$$

$g$  = gravity acceleration

$F_f$  = frictional force  $\approx C \frac{dz}{dt}$

$F_d$  = driving force (external and/or internal)  
 $= f \cos \omega t$

$M$  =  $M_1 + A(L - z_o)\rho$

$M_1$  = mass of concrete slab + magnets

$L$  = distance of slab to bedrock (assumed constant)

$\rho$  = density of soil

Substitution of the force  $F$  in Hooke's law gives

$$\frac{z - z_o}{z_o} = - \frac{Mg - M \frac{d^2 z}{dt^2} - C \frac{dz}{dt} + f \cos \omega t}{AE}$$

Setting  $z - z_0 + \frac{Mgz_0}{AE} = y$ ,  $\frac{dz}{dt} = \frac{dy}{dt}$  and

$$\frac{d^2z}{dt^2} = \frac{d^2y}{dt^2} \text{ we obtain}$$

for  $z_0 = L$  ( $M = M_1$ )

$$\frac{d^2y}{dt^2} + \lambda \frac{dy}{dt} + \Omega^2 y = \varepsilon \cos \omega t.$$

where  $\lambda = \frac{C}{M_1}$ ,  $\Omega^2 = \frac{AE}{M_1 L}$  and  $\varepsilon = \frac{f}{M_1}$ .

The solution of this equation can be written in the form

$$y = \frac{\varepsilon}{\sqrt{(\Omega^2 - \omega^2)^2 + (\lambda\omega)^2}} \cos(\omega t - \phi), \quad \tan \phi = \frac{\lambda\omega}{\Omega^2 - \omega^2}$$

We see that at resonance ( $\omega = \Omega$ ), the amplitude is limited only by the frictional damping ( $\lambda$ ). Knowing  $E$ ,  $L$ ,  $f$  and  $\lambda$  will give a rough estimate of the effects of vibrational disturbances.

In the foregoing treatment, the soil underneath the foundation is approximated by a pile. A more realistic result may be obtained by considering the soil as an "elastic" half-space. The weight of the concrete slab and the magnets will compress the soil and introduce vertical and horizontal strain. The static displacement is given by

$$\Delta z = \frac{M_1 g (1 - \mu)}{4G\sqrt{A/\pi}}$$

Here,  $\mu$  is the Poisson's ratio  $\frac{\text{horizontal strain}}{\text{vertical strain}}$  and  $G = \frac{E}{2(1+\mu)}$  is the shear modulus.

Comparing  $\Delta z$  with the corresponding displacement for the pile approximation  $\frac{M_1 g \ell}{AE}$  we obtain the effective spring constant  $\frac{4G}{1-\mu} \sqrt{\frac{A}{\pi}}$  and the resonant frequency  $\Omega^2 = \frac{4G}{M_1(1-\mu)} \sqrt{\frac{A}{\pi}}$ .

To estimate the damping, we assume that the damping constant  $C$  is proportional to the area  $A$ , the density  $\rho$  and the wave velocity  $\sqrt{\frac{G}{\rho}}$ . To make the ratio:

$\frac{\text{spring constant}}{\text{damping constant}}$  independent of the Poisson's ratio, we set

$$C = \frac{A\sqrt{G\rho}}{1-\mu} \text{ or } \lambda = \frac{A\sqrt{G\rho}}{M_1(1-\mu)}.$$

Assuming:  $\rho = 2 \times 10^3 \text{ kg/m}^3$

$$G = 400 \text{ kg/cm}^2 \approx 4 \times 10^7 \text{ N/m}^2$$

$$A = 5 \text{ m}^2$$

$$M_1 = 1.2 \times 10^4 \text{ kg}$$

$$\mu = 0.3$$

we obtain  $\Omega = 155 \text{ sec}^{-1}$  ( $F \approx 25 \text{ Hz}$ )

$$\lambda = 168 \text{ sec}^{-1}$$

Substitutions of  $\Omega$  and  $\lambda$  in  $A_y = \frac{\epsilon}{\sqrt{(\Omega^2 - \omega^2)^2 + (\lambda\omega)^2}}$  we find at resonance ( $\Omega = \omega$ )

$$A_y = 4 \text{ } \mu\text{m for } \epsilon = 0.01 \text{ g} \approx 0.1 \text{ m/sec}^2.$$